



December 8, 2003

DESIGN MEMORANDUM No. 03-19
POLICY CHANGE

TO: All Design, Operations, and District Personnel, and Consultants

FROM: /s/ Anthony L. Uremovich
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SUBJECT: Drainage in Curbed Section

COMPLEMENTS: *Indiana Design Manual* Chapters Fifty-three and Fifty-five

EFFECTIVE: April 20, 2004, Letting

In Chapters Fifty-three and Fifty-five it is stated that the minimum profile grade in a curbed section is $\pm 0.3\%$. Additional consideration should be given to minimum grades in curbed superelevation transition areas to avoid drainage problems. The following two criteria will alleviate such problems.

1. A minimum profile grade of $\pm 0.5\%$ should be maintained through a superelevation transition section.
2. A minimum edge of pavement grade of $\pm 0.5\%$ should be maintained through a superelevation transition section. The equations to be considered for this criterion are as follows:

$$G \leq -\Delta^* - 0.5 \quad [\text{Equation 03-19.1}]$$

$$G \geq -\Delta^* + 0.5 \quad [\text{Equation 03-19.2}]$$

$$G \leq \Delta^* - 0.5 \quad [\text{Equation 03-19.3}]$$

$$G \geq \Delta^* + 0.5 \quad [\text{Equation 03-19.4}]$$

$$\Delta^* = \frac{wne_d}{L_r} \quad [\text{Equation 03-19.5}]$$

where,

G = profile grade, %;
 Δ^* = effective maximum relative gradient, %;
 w = width of one traffic lane, m (typically 3.6)
 n = number of lanes rotated;
 e_d = design superelevation rate, %;
 L_r = length of superelevation runoff, m.

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EXAMPLE 03-19.1

To illustrate the combined use of the two criteria, consider the following:

$\Delta^* = 0.65\%$ in the transition section

Criterion 1 excludes grades between -0.5% and $+0.5\%$.

Criterion 2 excludes grades between -1.15% (via Equation 03-19.1, where $G \leq -0.65 - 0.5$, or -1.15), and -0.15% (via Equation 03-19.2, where $G \geq -0.65 + 0.5$, or -0.15). Also,

Criterion 2 excludes grades between $+0.15\%$ (via Equation 03-19.3, where $G \leq +0.65 - 0.5$, or $+0.15$), and $+1.15\%$ (via Equation 03-19.4, where $G \geq +0.65 + 0.5$, or $+1.15$).

Therefore, the profile grade within the transition must be outside the range of -1.15% to $+1.15\%$ in order to satisfy both criteria and provide adequate pavement surface drainage.

See the AASHTO *A Policy on Geometric Design of Highways and Streets*, 2001, pp. 190-91.